THE INFLUENCE OF THE THICKNESS OF A MINERAL LAYER OF GRANITE AND MARBLE ON THE MECHANICAL PROPERTIES OF A BILAYER MATERIAL: CASE OF THE GRANITOS

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ABSTRACT

The present work is dedicated to the study of the mechanical properties of a bilayer material. This material consists of a substrate in mortar and a mineral layer in granite and marble. The mixture of these two constituents of different characteristics gives a material whose properties will vary depending on the density of each constituent. The standardized testing on the sand and the bend tests three points and compression are among other methods used for the evaluation of the mechanical characteristics of the specimens of
4 cm × 4 cm × 16 cm dimensions.

The results of these tests show that samples of the mineral layer in granite with a thickness of 1.7 cm and that of 1.4 cm in marble get good bending resistance three points respectively equal to 10.63 MPa and 10.3 MPa. As for the compression tests, it appears that the compressive resistance increases with the thickness of the mineral layer but evolves in reverse with the rate of water absorption of these materials. The best resistance in compression obtained with the samples having the thickness of 2 cm of the granite and marble mineral layers are respectively 24.47 MPa and 24.07 MPa. In addition, for this same thickness, the Bilayers offer a better rate of water absorption.

Keywords: Granite, marble, bilayer material, mechanical resistance, water absorption.
water absorption.

NOMENCLATURE

LERGC : Test and Research Laboratory in Civil Engineering
UAC : Université of Abomey Calavi
SONEB : National Water Company of Benin
NOCIBE : New Cement Factory of Benin
M_S : Mass of sand (g)
M_C : Mass of cement (g)
M_E : Mass of rwate (g)
R_f : Maximum resistance to flexion (MPa)
b : Side of prism square section (mm)
F_f : Load applied in the middle of the prism at break (N)
l : Distance between supports (mm)
Rc: Compressive maximum resistance at 28 days (MPa)
Fc: Maximum load at break (N)
W : Absorption rate of water in (%)
m_1: The dry sample weight
m_2: The mass of saturated the sample water

N : Number of test tubes

$\overline{X}$: Resistance average to flexion or compressive in (MPa) or absorption rate of water in (%)

X_i : Resistance in flexion or compression in (MPa) or the water absorption rate in (%) of each test tube.

1. INTRODUCTION

Composite materials have seen a very significant advance in recent decades. The basic idea is to combine two or more compatible materials in order to achieve remarkable compromises between the properties of materials while taking advantage of the qualities of the components. These materials have excellent characteristics and have important advantages over traditional materials.

Among the most commonly used composite materials are two-layer materials. Cement / Granitos bilayers are inexpensive and environmentally friendly materials that can be produced using relatively simple technology. They are mainly used in the building for non-structural applications, as interior and exterior coatings, light partitions, tiles, square tiles, pavers, screeds, noise and fire barriers ... (Moslemi, 1999). These materials offer a good lightness, a good behavior with humidity, good resistance to fire and shocks with interesting mechanical and thermal performances ....

Many parameters affect the mechanical behavior of these bilayer structures:

- The relative thicknesses of the different layers;
- Stacking sequences;
- The nature of the constituent materials (density, physical and mechanical properties).

The bibliographical reference relating to granitic bilayer materials suffers a lot of insufficiencies. Meanwhile, for some years, some work has been done on composite materials. Thus, the work of (Pouteau, Chabot and De Larrard, 2002) and (Pouteau, 2004) focused on a 4-point flexural bending test of bilayer structures. The efforts at the interface between the layers of the composite materials have been analyzed by (Chabot A, 1997). (Romain BRAULT, 2013) studied the mechanical behavior of composite materials by volume correlation. The work of (Taoukil, Albouardi, Ajzoui and Ezbakhe, 2011) and (Aouadja, Mimoune, and Laquerbe, 1995) relate respectively to mechanical and water characterization and experimental study of concrete with wood residues. In addition, the characterization of a mineral bilayer material produced by (Toukourou, Fagla, Prodjinonto and Bello, 2015) has shown that at 20% of coarse sand, granitic bilayers offer good resistance in three points and in compression with a normal water absorption rate. Moreover, these results obtained by these authors are practically the same as those obtained with 30% coarse sand content.

However, knowledge in the field of bilayer materials including granitos is still limited and studies must be conducted to better know and improve the properties of these materials. The present study is therefore part of this logic and aims to study the influence of the thickness of the granite and marble mineral layer on the mechanical characteristics of such materials.

2. MATERIAL AND METHODS

2.1. Equipment

The materials used in the manufacture of bilayers are: sand, cement, water and granitos. The following paragraphs describe the characteristics of each of these constituents.

2.1.1. Sand

The sand used in this work comes from the region of Agamè, commune of Bonou; region located in the south of Benin, in the department of Ouémé. This sand, washed with tap water, is dried in an oven at 105 ° C until stabilization of the mass and then sieved in two different granular classes: Medium sand and coarse sand composed of grains, respectively with the dimensions ranging from 0.4 mm to 2 mm and from 2 mm to 6.3 mm. The sand used for the preparation of the substrate is composed of 30% of coarse sand and 70% of medium sand (Toukourou, Fagla, Prodjinonto and Bello, 2015).

2.1.2. Cement

The porland cement CPJ 35 of type CEM II/B-LL.42,5R of the NOCIBE is used because of its relatively fast setting.

2.1.3. Granitos

The granitos used are crushed granite and marble respectively from the town of zangnanado and abomey, two regions of the southern benin located in the department of zou (fig. 1).

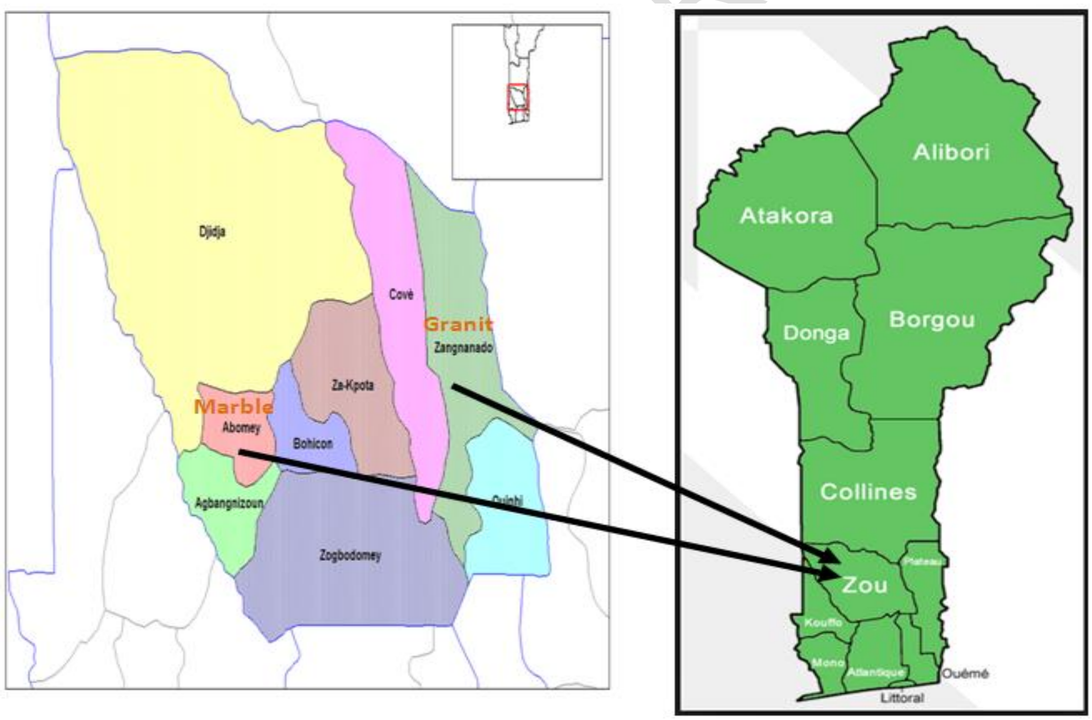

Fig. 1 : Geographical location of rock sampling sites

2.1.4. Water

The water used is that of SONEB collected at UAC. It is supposed to be drinkable and contains no impurities harmful to the mechanical characteristics of bilayer materials.

2.2. Methods

In order to carry out the mechanical tests, the parallelepiped shaped samples were made after having characterized the sand.

2 .2.1. The physical test on the sand

Before the preparation of the substrate on which the granito must rest, the sand was initially characterized by LERGC. These physical tests are:

- The particle size analysis
- the equivalent of sand
- the actual pre-dried density (specific weight),
- the bulk density
- the water content.

2 .2.2. The samples making

The preparation of the specimens began with the preparation of the substrate while varying its thickness and respecting the ratios of the dosage:

$$\frac{M_C}{M_S} = \frac{1}{3} \quad (1)$$

$$\frac{M_E}{M_C} = \frac{1}{2} \quad (2)$$

After this step, the granite coating is placed so as to have a final bilayer sample of 4 cm thick (fig. 2). Once the bilayer was obtained, we immersed it in water for 28 days. The 28 days reached, the surface of the sample out of the water is sanded (fig. 3). The bilayers then obtained, the step of the tests of three-point bending and compression at 28 days on the test tubes is carried out.

Fig. 2: coating laying **Fig. 3: sanding operation**

2.2.3. <u>The three-point bending test</u>

The press used for our tests is a hydraulic press with digital display (fig. 4). The test piece is centered along its length while presenting the face of the mineral layer to the effort. Then, a normal force is applied to this face and the value of the maximum load at break is noted after each test.

The flexural strength of the specimens was determined by the relationship established by (BAILON and DORLOT, 2000):

$$R_f = \frac{1{,}5 \times F_f \times l}{b^3} \quad (3)$$

Fig. 4: The three-point bending and compression testing equipment on the test specimen

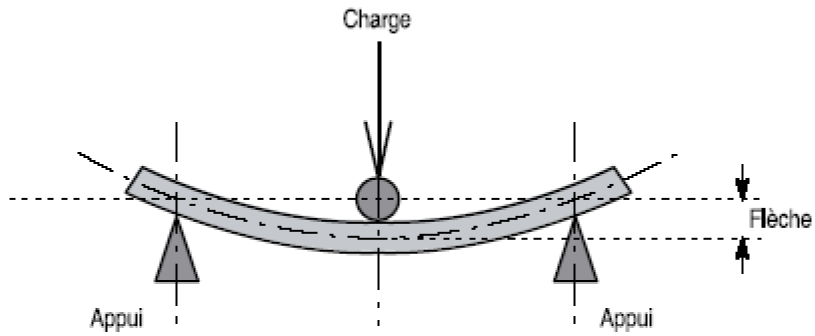

Fig. 5 : The three-point bend test devices on the test specimen

<u>2.2.4. The compression test</u>

The procedure adopted is the compression on each half-test piece resulting from the three-point bending test with the same apparatus, the two halves not being always of regular shape.

The compressive strength is obtained by the formula:

$$\boldsymbol{R_c} = \frac{\boldsymbol{F_c}}{\mathbf{1600}} \qquad (4)$$

2.2.5. **The bilayer water absorption test**

The samples, initially dried and weighed with a precision scale of 0.1 g, were immersed in water and then left after 24 hours of total immersion. Wiped with a damp cloth, these samples were weighed again. The rate of water absorption is determined by the following formula:

$$\mathrm{W}\ (\%) = \frac{\mathrm{m_2} - \mathrm{m_1}}{\mathrm{m_1}} \times 100 \qquad (5)$$

2.2.6. <u>The assessment of uncertainties</u>

The standard deviation is determined from the following formula:

$$\mathrm{E} = \sqrt{\frac{1}{\mathrm{N}} \Sigma(\bar{\mathrm{x}} - \mathrm{xi}\ )^2} \qquad (6)$$

The uncertainty is deduced from the following relation:

$$\mathrm{I}(\%) = \frac{\mathrm{E}}{\mathrm{E_{moyenne}}} \times 100 \qquad (7)$$

3. RESULTS AND DISCUSSION

3.1. The influence of the thickness of the granite layer and marble on the 3 point flexural strength

Tables 1 and 2 summarize the results of the three-point bending resistances obtained on the different samples. These are the average values found on the different test pieces. These values made it possible to draw curves representing the variation of the bending strength as a function of the thickness of the granite layer and the marble (Figures 6). It can be seen that there are no significant differences between the 28 day flexural rupture strength values of granite bilayers and those of marble bilayers of the same thickness.

In addition, we note that these curves first decrease with the thicknesses (0.8 cm and 1.1 cm) of the marble and granite layer and then oscillate as the thickness of the mineral layer increases. . The decrease followed by growth could be explained respectively by an excess of binder in the coating causing the effective nonparticipation of the granitos and a deficiency of binder to coat these granitos which represent one of the factors of the resistance of the bilayer material.

Such behavior of the samples is similar to that of bilayer materials (TOUKOUROU et al, 2015), fragile materials (BAILON and DORLOT, 2000).

Table 1: The summary of the 3-point flexural test results of granite bilayers

Thickness of the mineral layer (cm)	Flexural strength (MPa)	Spread type (MPa)	Uncertainty (%)	Average uncertainty (%)
0.8	9.93	0.83	8.36	
1.1	9.47	0.37	3.91	
1.4	9.9	0.67	6.77	5.44
1.7	10.63	0.31	2.92	
2	9.7	0.51	5.26	

Table 2: The summary of the 3-point flexural test results of marble bilayers

Thickness of the mineral layer (cm)	Flexural strength (MPa)	Spread type MPa)	Uncertainty (%)	Average uncertainty (%)
0.8	9.9	0.22	2.22	
1.1	8	0.43	5.38	
1.4	10.27	0.45	4.38	5.66
1.7	9.37	0.74	7.90	
2	9.4	0.79	8.40	

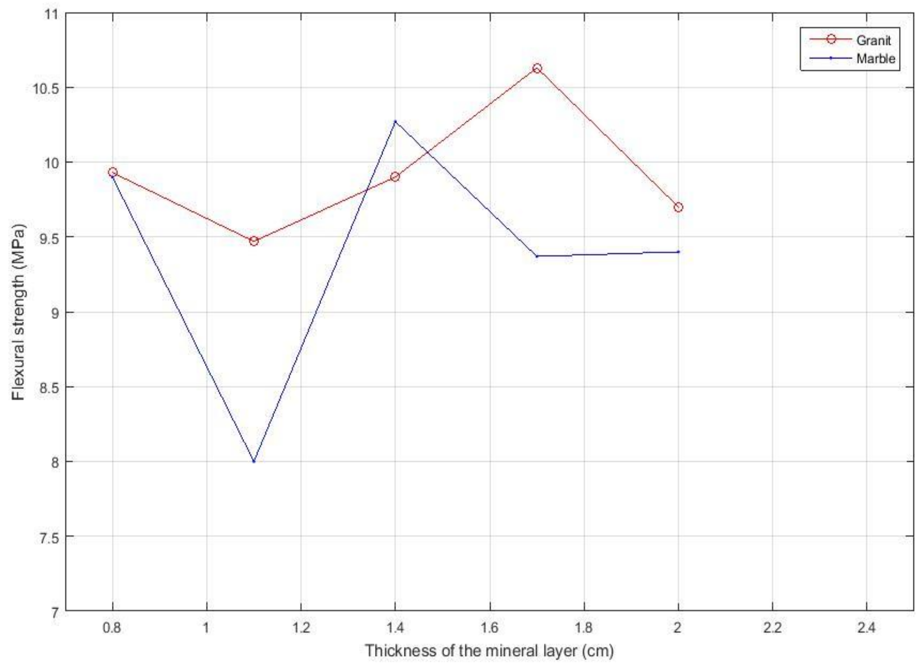

Fig. 6: The influence of granite layer and marble thickness on 3-point flexural strength at 28 days

3.2. The influence of granite layer and marble thickness on compressive strength at 28 days

The results of the 28-day compressive strengths of the different samples are given in Tables 3 and 4. Figure 7 shows the 28 day compressive strength variation curves of the two categories of bilayers as a function of the thickness of the mineral layer of granite and marble.

It can be seen in this figure that the compressive strength increases in the same direction as the thickness of the granite and marble mineral layer. As a result, an increase in the thickness of the mineral layer is synonymous with an increase in the volume of rock aggregates, whose compressive strength would drive that of the composite material. Moreover, it is important to note that these bilayers exhibit for this variation in the thickness of the mineral layer a different behavior in flexion than in compression. In addition, these materials are rigid in compression than in flexion.

Table 3: The summary of 28 day compression test results for granite bilayers

hickness of the mineral layer (cm)	Compressive strengh (MPa)	Spread type (MPa)	Uncertainty (%)	Average uncertainty (%)
0.8	13.93	0.29	2.08	
1.1	15.67	0.58	3.70	
1.4	16.87	0.93	5.51	4.81
1.7	22.70	1.27	5.59	
2	24.47	1.76	7.19	

Table 4: The summary of 28 day compression test results for marble bilayers

hickness of the mineral layer (cm)	Compressive strengh (MPa)	Spread type(MPa)	Uncertainty (%)	Average uncertainty (%)
0.8	13.23	0.53	4.01	
1.1	14.27	0.59	4.13	
1.4	17.1	1	5.85	4.60
1.7	23.63	1.11	4.70	
2	24.23	1.05	4.33	

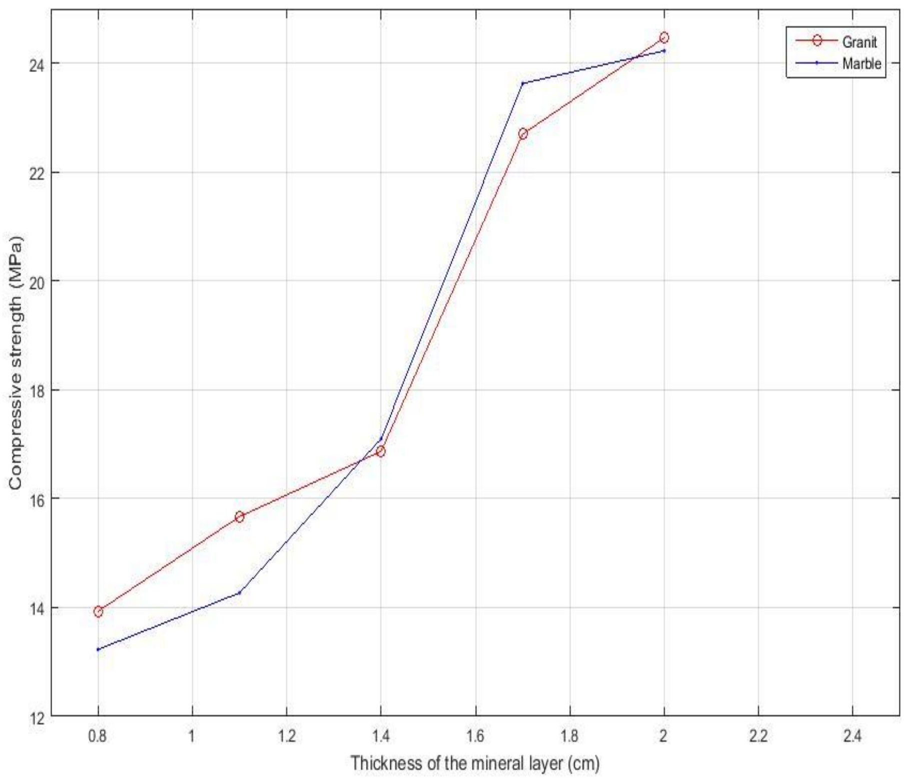

Fig. 7: The influence of granite and marble layer thickness on compressive strength at 28 days.

3.3. The influence of the thickness of the granite layer and marble on the water absorption rate of bilayers

Fig 8 gives an overview of the variation in the rate of water absorption by total immersion of the specimens as a function of the thickness of the layer of granite and marble. These curves are obtained from the results synthesized in Tables 5 and 6. It can be seen that the water absorption rate of the various samples decreases as the thickness of the mineral layer increases and varies between 3.09% and 5, 25%. This decrease is all the stronger as the thickness of the mineral layer is important. This observation can be justified by a low porosity of aggregates including rocks used on the one hand or by a high level of binder in the mixture of the mineral layer, thus making the bilayer material tight on the other hand.

Table 5: The summary of water Absorption study results for granite bilayers

hickness of the mineral layer (cm)	Absorption rate of water (%)	Spread type(MPa)	Uncertainty (%)	Average uncertainty (%)
0.8	4.15	0.26	6.27	
1.1	3.50	0.11	3.14	
1.4	3.35	0.18	5.37	4.60
1.7	3.25	0.13	4.00	
2	3.09	0.13	4.21	

Table 6: The summary of water absorption study results of marble bilayers

hickness of the mineral layer (cm)	Absorption rate of water (%)	Spread type(MPa)	Uncertainty (%)	Average uncertainty (%)
0.8	5.25	0.3	5.71	
1.1	4.54	0.15	3.30	
1.4	4.34	0.13	3.00	4.23
1.7	4.22	0.18	4.27	
2	3.96	0.19	4.80	

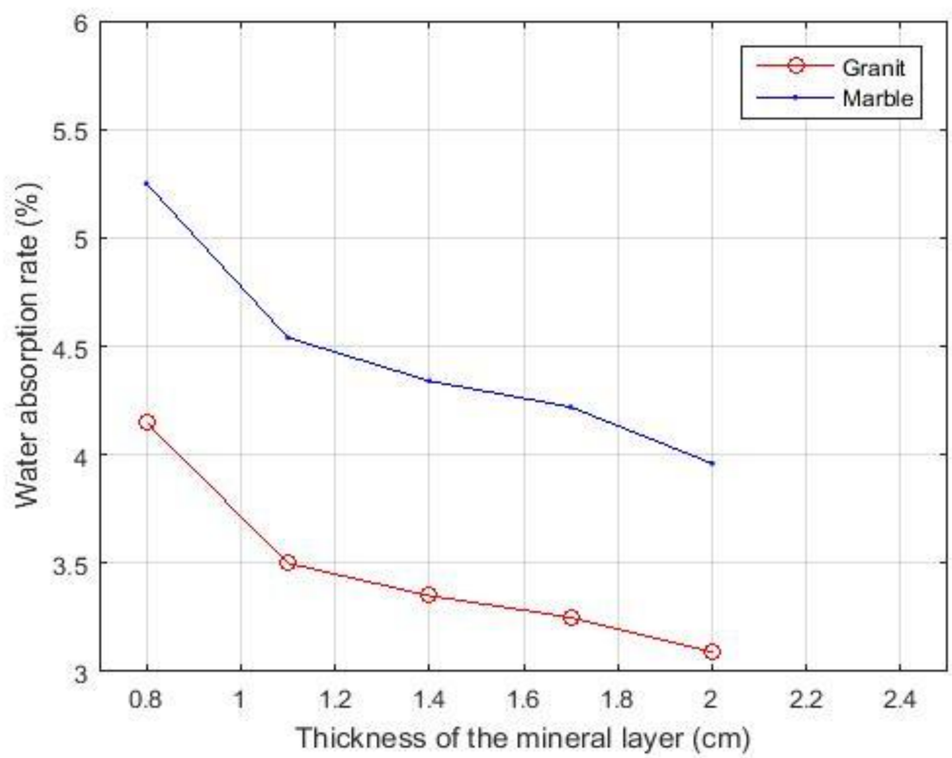

Fig. 8: The influence of the thickness of the granite layer and marble on the rate of water

3.4. The evolution of the compressive strength in terms of the water absorption rate of the bilayers.

Fig. 9 gives the evolution of the compressive strength in terms of the water absorption rate of the bilayers. A decrease in the compressive strength is noted for an increase in the water absorption rate, and therefore in the open porosity of the bilayers. Since these bilayer materials consist of the substrate and a mineral granite and marble coating, this reduction could be justified by an increase in the amount of pores in the substrate whose thickness increases when that of the mineral layer decreases.

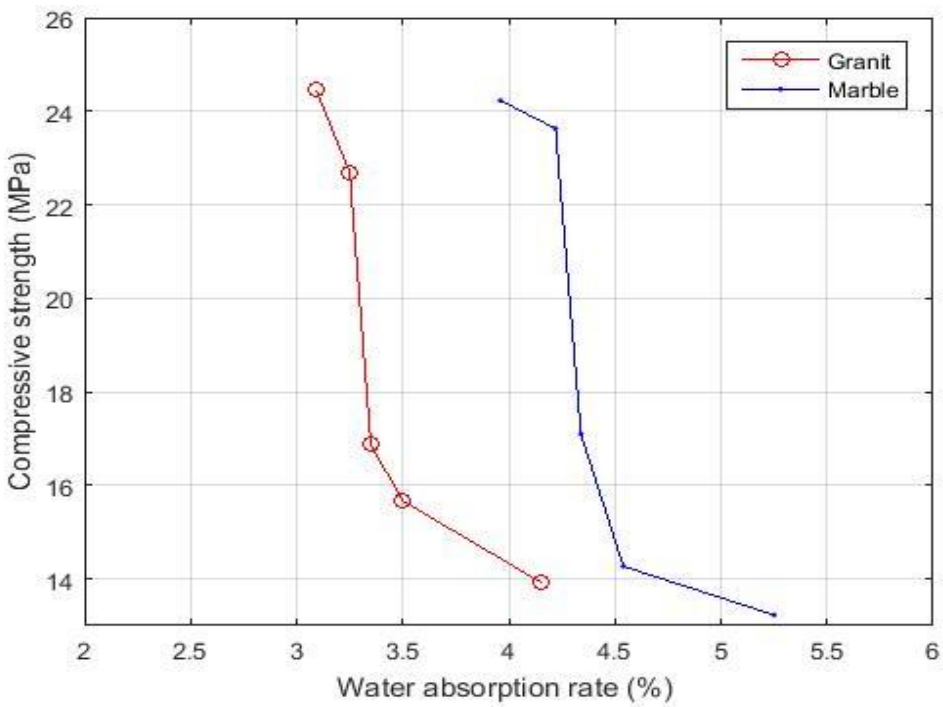

Fig. 9: The influence of compressive strength on the open porosity of granite bilayers.

4. CONCLUSION

The materials used in the building industry must have, among other things, certain mechanical properties to ensure the structural durability of the building.
The studies carried out as part of this work whose objective is the valorization of local building materials in Benin with a view of improving their performance in terms of thermomechanical characteristics, have focused on bilayer materials, especially tiles coated with granitos. The results obtained showed that:

- Granite-clad tiles are suitable for both interior and exterior cladding in our premises;
- For a thickness of 2 cm of the granite and marble mineral layer, the two-ply materials offer good compression strength at 28 days respectively equal to 24.47 MPa and 24.23 MPa with a normal water absorption rate;

- For a mineral layer of 1.7 cm of granite and 1.4 cm of marble, these two-layer materials have good three-point bending strengths respectively equal to 10.63 MPa and 10.27 MPa.

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